

The finite element method

III

In FEM II we derived finite element equations from PVD:

$$\delta W_E^e = \delta W_I^e$$

and obtained:

$$K_{ij}^e U_j^e = R_i^e \quad i, j = 1, \dots, n$$

where:

n : number of element nodal points
 U_j^e : element nodal displacements

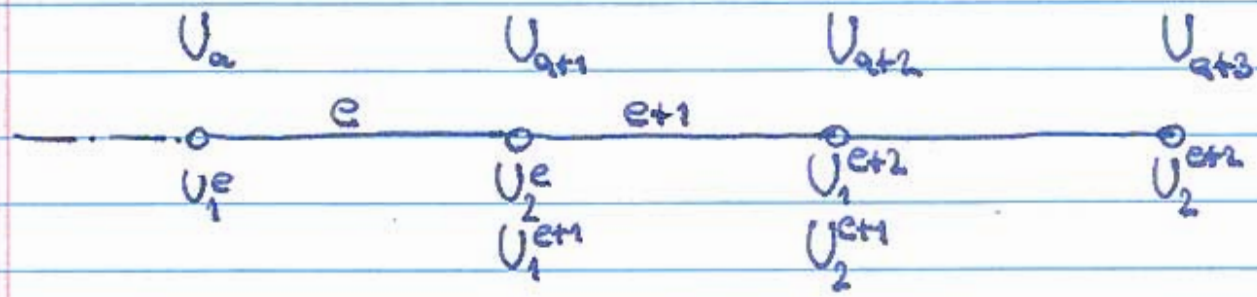
$$K_{ij}^e = \int_{x_f^e}^{x_n^e} E A \frac{d\phi_i^e}{dx} \frac{d\phi_j^e}{dx} dx \quad : \text{element stiffness matrix}$$

$$R_i^e = \int_{x_f^e}^{x_n^e} q(x) \phi_i^e dx + P_j^e \quad : \text{element nodal force vector}$$

- Exercise: derive K_{ij}^e , R_i^e for linear and quadratic interpolation for the case of uniform cross section (A), Young's modulus (E) and distributed load ($q(x) = q_0$).

Finite element assembly

- Each finite element is connected to its neighbors at its end nodes.
- As before define a global numbering (identification) for all the nodes



For linear elements:

$$U_1^1 = U_1, U_2^1 = U_2 \neq U_1^2, U_2^2 = U_3 = U_1^3 \dots$$

$$U_2^e = U_{e+1} = U_1^{e+1}$$

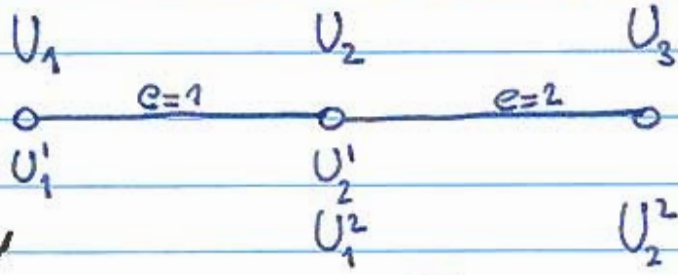
- Concept question: Expression for the local-global nodal mapping for the quadratic element mesh.

The assembly of the element equations is based on the satisfaction of the variational principle for the whole system:

$$\delta \Pi = \sum_{i=1}^{n+1} \frac{\partial \Pi}{\partial U_i} \delta U_i = 0$$

where Π is the sum of the element values Π^e over the mesh elements.

Example: Mesh with two linear elements



strain energy

$$U^e = \frac{1}{2} \{U^e\}^T [K^e] \{U^e\}$$

why?

1x1 1xn nxn nx1

$$\epsilon^e = \frac{U_2^e - U_1^e}{L^e}, \quad \sigma^e = E \epsilon^e, \quad U_0^e = \frac{1}{2} E (\epsilon^e)^2$$

$$U^e = A^e L^e U_0^e = \frac{A^e \cancel{L^e} E (U_2^e - U_1^e)^2}{2} = \frac{A^e E^e (U_2^e - U_1^e)^2}{2 L^e}$$

We had obtained K^e as:

$$K^e = \frac{A^e E^e}{L^e} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix}$$

$$\begin{aligned}
 \underline{\frac{1}{2} \{U^e\}^T [K] \{U^e\}} &= \frac{A^e E^e}{2 L^e} \{U_1^e \ U_2^e\} \begin{bmatrix} 1 & -1 \\ -1 & 1 \end{bmatrix} \begin{Bmatrix} U_1^e \\ U_2^e \end{Bmatrix} \\
 &= \frac{A^e E^e}{2 L^e} \{U_1^e - U_2^e \quad U_2^e - U_1^e\} \begin{Bmatrix} U_1^e \\ U_2^e \end{Bmatrix} \\
 &= \frac{A^e E^e}{2 L^e} (U_1^e - U_2^e)^2 = \underline{U^e} \text{ q.e.d.}
 \end{aligned}$$

Replacing local with global numbering:

strain energy e=1

$$\begin{aligned}
 \underline{U^1} &= \frac{1}{2} \{U_1^1 \ U_2^1\} \begin{bmatrix} K_{11}^1 & K_{12}^1 \\ K_{21}^1 & K_{22}^1 \end{bmatrix} \begin{Bmatrix} U_1^1 \\ U_2^1 \end{Bmatrix} \\
 &= \frac{1}{2} \{U_1 \ U_2 \ U_3\} \begin{bmatrix} K_{11}^1 & K_{12}^1 & 0 \\ K_{21}^1 & K_{22}^1 & 0 \\ 0 & 0 & 0 \end{bmatrix} \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \end{Bmatrix}
 \end{aligned}$$

strain energy e=2

$$\begin{aligned}
 \underline{U^2} &= \frac{1}{2} \{U_1^2 \ U_2^2\} \begin{bmatrix} K_{11}^2 & K_{12}^2 \\ K_{21}^2 & K_{22}^2 \end{bmatrix} \begin{Bmatrix} U_1^2 \\ U_2^2 \end{Bmatrix} \\
 &= \frac{1}{2} \{U_1 \ U_2 \ U_3\} \begin{bmatrix} 0 & 0 & 0 \\ 0 & K_{11}^2 & K_{12}^2 \\ 0 & K_{21}^2 & K_{22}^2 \end{bmatrix} \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \end{Bmatrix}
 \end{aligned}$$

$$U^{TOTAL} = U^1 + U^2$$

$$= \frac{1}{2} \{ U_1 \ U_2 \ U_3 \} \begin{bmatrix} K_{11}^1 & K_{12}^1 & 0 \\ \text{sym} & K_{22}^1 + K_{11}^2 & K_{12}^2 \\ & & K_{22}^2 \end{bmatrix} \begin{Bmatrix} U_1 \\ U_2 \\ U_3 \end{Bmatrix}$$

$$\Rightarrow K^{TOTAL} = \sum_{e=1}^E K^e$$

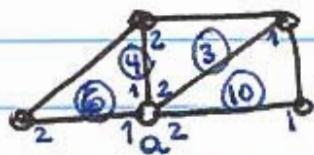
but here the "sum" sign has a special meaning: "assembly" which involves proper

placement of the element stiffness matrix coefficients in the global matrix.

Remarks on assembly of global matrix

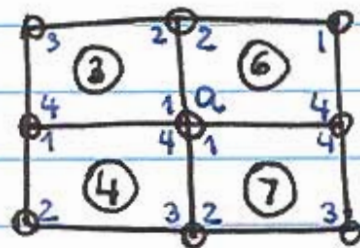
- Diagonal element corresponding to global node "i" gets contributions from all the elements writing to it.

Trusses, beams:



$$K_{aa} = k_{22}^{(3)} + k_{11}^{(4)} + k_{11}^{(6)} + k_{22}^{(10)}$$

Plane elasticity



$$K_{aa} = k_{11}^{(7)} + k_{33}^{(6)} + k_{11}^{(3)} + k_{44}^{(4)}$$

(x-dir)

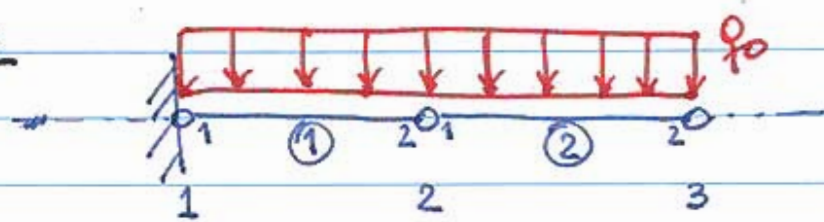
- $K_{ij} = 0$ if node "i" and "j" are in different elements
- K_{ij} preserves the symmetry of the element K_{ij}^e

Assembly of global force vector R_i

Given $R_i^e, e=1, \dots, E$

Find: $R_i = \sum_{e=1}^E R_i^e$

Example:



$$R^e = \frac{L^e q_0}{2} \begin{Bmatrix} 1 \\ 1 \end{Bmatrix}$$

$$R = R^1 + R^2 = \begin{Bmatrix} 1 \\ 1 \\ 0 \end{Bmatrix} \frac{L^1 q_0}{2} + \begin{Bmatrix} 0 \\ 1 \\ 1 \end{Bmatrix} \frac{L^2 q_0}{2}$$

$$R = \frac{q_0}{2} \begin{Bmatrix} L^1 \\ L^1 + L^2 \\ L^2 \end{Bmatrix}$$